

Name:

Enrolment No:



UNIVERSITY OF PETROLEUM AND ENERGY STUDIES

End Semester Examination, May 2021

Course: Water Resources Engineering

Program: B Tech Civil Engineering

Course Code: CIVL 3023

Semester: VI

Time: 3 hours

Max. Marks: 100

Instructions: Attempt all the questions

SECTION A

1. Each Question will carry 5 Marks

2. Instruction: Complete the statement / Calculate the correct answer(s)

S. No.		Marks	CO
Q1	The normal annual rainfall at stations A, B, C, and D in a basin are 80.97, 67.59, 76.28 and 92.01 cm respectively. In the year 1975, the station D was inoperative and the stations A, B and C recorded annual precipitations of 91.11, 72.23 and 79.89 cm respectively. Estimate the rainfall at station D in that year.	5	CO1
Q2	A reservoir has an average area of 50 km² over an year. The normal annual rainfall at the place is 120 cm and the class A pan evaporation is 240 cm . Assuming the land flooded by the reservoir has a runoff coefficient of 0.4 , estimate the net annual increase or decrease in the streamflow as a result of the reservoir.	5	CO1
Q3	The peak of a flood hydrograph due to a 6-h storm is 470 m ³ /s. The mean depth of rainfall is 8.0 cm. Assume an average infiltration loss of 0.25 cm/h and a constant base-flow of 15 m ³ /s and estimate the peak discharge of the 6-h unit hydrograph for this catchment.	5	CO2
Q4	Find the delta for a crop when its duty is 4.32 km ² per m ³ /s on the field, the base period of this crop is 60 days.	5	CO3
Q5	Determine the time to irrigate a strip of land 0.1 Ha in an area from a tube-well with a discharge of 0.2 cumecs. The infiltration capacity of the soil may be taken as 0.5 cm/hr, and the average depth of flow on the field as 0.1 m.	5	CO3
Q6	Ordinates of the one hour unit hydrograph of a basin at one-hour intervals are 5, 8, 5, 3 and 1 m ³ /s. Calculate the watershed area represented by this unit hydrograph.	5	CO2

SECTION B

1. Each question will carry 10 marks

2. Instruction: Write short / brief notes

<p>Q7</p>	<p>A catchment has four sub-areas. The annual precipitation and evaporation from each of the sub-areas are given below. Assume that there is no change in the groundwater storage on an annual basis and calculate for the whole catchment the values of annual average (i) precipitation, and (ii) evaporation. What are the annual runoff coefficients for the sub-areas and for the total catchment taken as a whole?</p> <table border="1" data-bbox="256 401 1239 793"> <thead> <tr> <th>Sub-Area</th> <th>Area Mm²</th> <th>Annual Precipitation (mm)</th> <th>Annual Evaporation (mm)</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>10.7</td> <td>1030</td> <td>530</td> </tr> <tr> <td>B</td> <td>3</td> <td>830</td> <td>438</td> </tr> <tr> <td>C</td> <td>8.2</td> <td>900</td> <td>430</td> </tr> <tr> <td>D</td> <td>17</td> <td>1300</td> <td>600</td> </tr> </tbody> </table> <p>Also sketch the hydrological cycle shoeing these components.</p>	Sub-Area	Area Mm ²	Annual Precipitation (mm)	Annual Evaporation (mm)	A	10.7	1030	530	B	3	830	438	C	8.2	900	430	D	17	1300	600	<p>10</p>	<p>CO1</p>						
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<p>Q8</p>	<p>The ordinates of a 6-h unit hydrograph are as given below:</p> <table border="1" data-bbox="302 978 1167 1094"> <thead> <tr> <th>Time (h)</th> <th>0</th> <th>6</th> <th>12</th> <th>18</th> <th>24</th> <th>30</th> <th>36</th> <th>42</th> <th>48</th> <th>54</th> <th>60</th> <th>66</th> </tr> </thead> <tbody> <tr> <td>ordinate of 6-h UH (m³/s)</td> <td>0</td> <td>20</td> <td>60</td> <td>150</td> <td>120</td> <td>90</td> <td>66</td> <td>50</td> <td>32</td> <td>20</td> <td>10</td> <td>0</td> </tr> </tbody> </table> <p>If two storms, each of 1-cm rainfall excess and 6-h duration occur in succession, calculate the resulting hydrograph of flow. Assume base flow to be uniform at 10 m³/s.</p>	Time (h)	0	6	12	18	24	30	36	42	48	54	60	66	ordinate of 6-h UH (m ³ /s)	0	20	60	150	120	90	66	50	32	20	10	0	<p>10</p>	<p>CO2</p>
Time (h)	0	6	12	18	24	30	36	42	48	54	60	66																	
ordinate of 6-h UH (m ³ /s)	0	20	60	150	120	90	66	50	32	20	10	0																	
<p>Q9</p>	<p>For a river, the estimated flood peaks for two return periods by the use of Gumbel's method are as follows:</p> <table border="1" data-bbox="383 1362 1110 1575"> <thead> <tr> <th>Return Period (Years)</th> <th>Peak Flood (cumecs)</th> </tr> </thead> <tbody> <tr> <td>100</td> <td>435</td> </tr> <tr> <td>50</td> <td>395</td> </tr> </tbody> </table> <p>What flood discharge in this river will have a return period of 1000 years?</p>	Return Period (Years)	Peak Flood (cumecs)	100	435	50	395	<p>10</p>	<p>CO2</p>																				
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100	435																												
50	395																												
<p>Q10</p>	<p>Compute the depth and frequency of irrigation required for a a certain crop with the data given below:</p> <ul style="list-style-type: none"> a) Depth of root zone = 1m b) Field Capacity = 22% c) Wilting point = 22% d) Consumptive use = 25 mm /day 	<p>10</p>	<p>CO3</p>																										

	<p>e) Efficiency of Irrigation = 10 %</p> <p>f) Apparent specific gravity of soil = 1.5</p> <p>Assume 50 % depletion of moisture before application of irrigation water at field capacity.</p>		
Q11	<p>With the help of a neat sketch explain the working of given below instruments:</p> <p>a) Double ring infiltrometer</p> <p>b) Class A pan evaporimeter</p>	5+5	CO1
SECTION-C			
<p>1. Each Question carries 20 Marks.</p> <p>2. Instruction: Write long answer.</p>			
Q12	<p>a) Design a stable canal section to carry-50 cumecs discharge at a slope of 0.25 m/km, having been given that $n = 0.0225$, and $m = 1.00$, where the symbols have their usual meaning.</p> <p>b) Compare Lacey's theory with Kennedy's theory.</p> <p>c) Design an irrigation channel section for the following data: Discharge = 30 cumecs Silt factor = 1.0 Side slopes = 0.5 :1.</p>	10+3+ 7	CO4
OR			
Q12	<p>a) A most efficient trapezoidal section is required to give a maximum discharge of 21.5 cumecs. The slope of the channel bottom is 1 in 50 m/km. Taking C as 100 (Chezy's constant), determine the dimensions of the channel. Also determine the value of Manning's n taking the velocity of flow as obtained for the channel by Chezy's Equation.</p> <p>b) Discuss the three regime conditions for canal design with respect to Lacey's theory.</p>	15+5	CO4